

25 Years
Previous Solved Papers

ESE 2024

UPSC ENGINEERING SERVICES EXAMINATION

Preliminary Examination

MECHANICAL ENGINEERING

Objective Solved Papers

Volume-II

- ✓ Topicwise presentation
- ✓ Thoroughly revised & updated

Also useful for

State Engineering Services Examinations, Public Sector Examinations
& Other Competitive Examinations





MADE EASY Publications Pvt. Ltd.

Corporate Office: 44-A/4, Kalu Sarai (Near Hauz Khas Metro Station), New Delhi-110016

Contact: 9021300500

E-mail: infomep@madeeasy.in

Visit us at: www.madeeasypublications.org

ESE-2024 : Preliminary Examination Mechanical Engineering : Volume-2 Topicwise Objective Solved Questions : (1999-2023)

© Copyright, by MADE EASY Publications Pvt. Ltd.

All rights are reserved. No part of this publication may be reproduced, stored in or introduced into a retrieval system, or transmitted in any form or by any means (electronic, mechanical, photo-copying, recording or otherwise), without the prior written permission of the above mentioned publisher of this book.

1st Edition : 2007
2nd Edition : 2008
3rd Edition : 2009
4th Edition : 2010
5th Edition : 2011
6th Edition : 2012
7th Edition : 2013
8th Edition : 2014
9th Edition : 2015
10th Edition : 2016
11th Edition : 2017
12th Edition: 2018
13th Edition: 2019
14th Edition: 2020
15th Edition: 2021
16th Edition: 2022

17th Edition: 2023

MADE EASY PUBLICATIONS PVT. LTD. has taken due care in collecting the data and providing the solutions, before publishing this book. In spite of this, if any inaccuracy or printing error occurs then MADE EASY PUBLICATIONS PVT. LTD owes no responsibility. MADE EASY PUBLICATIONS PVT. LTD will be grateful if you could point out any such error. Your suggestions will be appreciated.

© All rights reserved by MADE EASY PUBLICATIONS PVT. LTD. No part of this book may be reproduced or utilized in any form without the written permission from the publisher.

Director's Message



B. Singh (Ex. IES)

Engineering is one of the most chosen graduating field. Taking engineering is usually a matter of interest but this eventually develops into “purpose of being an engineer” when you choose engineering services as a career option.

Train goes in tunnel we don't panic but sit still and trust the engineer, even we don't doubt on signalling system, we don't think twice crossing over a bridge reducing our travel time; every engineer has a purpose in his department which when coupled with his unique talent provides service to mankind.

I believe *“the educator must realize in the potential power of his pupil and he must employ all his art, in seeking to bring his pupil to experience this power”*. To support dreams of every engineer and to make efficient use of capabilities of aspirant, MADE EASY team has put sincere efforts in compiling all the previous years' ESE-Pre questions with accurate and detailed explanation. The objective of this book is to facilitate every aspirant in ESE preparation and so, questions are segregated chapterwise and topicwise to enable the student to do topicwise preparation and strengthen the concept as and when they are read.

I would like to acknowledge efforts of entire MADE EASY team who worked hard to solve previous years' papers with accuracy and I hope this book will stand up to the expectations of aspirants and my desire to serve student fraternity by providing best study material and quality guidance will get accomplished.

B. Singh (Ex. IES)
CMD, MADE EASY Group

Contents

Sl.	Topic	Pages
1.	Strength of Materials and Engg. Mechanics	1-101
2.	Engineering Materials	102-162
3.	Mechanisms and Machines	163-268
4.	Design of Machine Elements	269-344
5.	Manufacturing Engineering	345-445
6.	Industrial Engineering and Maintenance Engg.	446-521
7.	Mechatronics and Robotics	522-542

UNIT

I

Strength of Materials and Engineering Mechanics

Syllabus

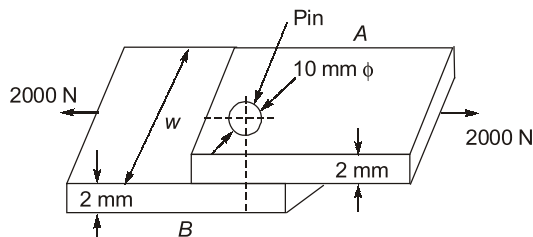
Analysis of System of Forces, Friction, Centroid and Centre of Gravity, Dynamics; Stresses and Strains-Compound Stresses and Strains, Bending Moment and Shear Force Diagrams, Theory of Bending Stresses-Slope and deflection-Torsion, Thin and thick Cylinders, Spheres.

Contents

Sl.	Topic	Page No.
1.	Stress and Strain	2
2.	Stress-Strain Relationship and Elastic Constants	10
3.	Principal Stresses and Strains and Mohr's Circle	10
4.	Thin Cylinder and Thick Cylinder	29
5.	Shear Force and Bending Moment Diagrams	38
6.	Bending of Beams, Shear Stress Distribution	49
7.	Torsion of Shafts	63
8.	Euler's Theory of Column	71
9.	Strain Energy and Thermal Stresses	78
10.	Deflection of Beams	84
11.	Combined Stresses	89
12.	Engineering Mechanics	91
13.	Miscellaneous	101



- 1.1 If permissible stress in plates of joint through a pin as shown in the figure is 200 MPa, then the width w will be



- (a) 15 mm (b) 20 mm
(c) 18 mm (d) 25 mm [ESE : 1999]

- 1.2 The state of plane stress in a plate of 100 mm thickness is given as

$$\sigma_{xx} = 100 \text{ N/mm}^2, \sigma_{yy} = 200 \text{ N/mm}^2$$

Young's modulus = 300 N/mm²

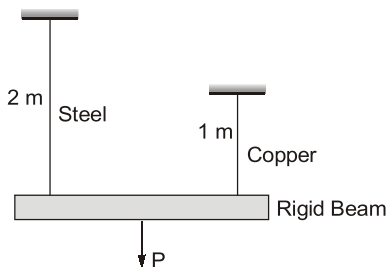
Poisson's ratio = 0.3

The stress developed in the direction of thickness is

- (a) zero (b) 90 N/mm²
(c) 100 N/mm² (d) 200 N/mm²

[ESE : 2000]

- 1.3 A rigid beam of negligible weight is supported in a horizontal position by two rods of steel and copper, 2 m and 1 m long having values of cross-sectional area 1 cm² and 2 cm² and E of 200 GPa and 100 GPa respectively. A load P is applied as shown in the figure below.

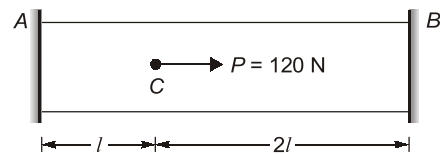


If the rigid beam is to remain horizontal then

- (a) the forces on both sides should be equal
(b) the force on copper rod should be twice the force on steel
(c) the force on the steel rod should be twice the force on copper

- (d) the force P must be applied at the centre of the beam [ESE : 2002]

- 1.4 A straight bar is fixed at edges A and B . Its elastic modulus is E and cross-section is A . There is a load $P = 120 \text{ N}$ acting at C . The reactions at the ends are



- (a) 60 N at A , 60 N at B
(b) 30 N at A , 90 N at B
(c) 40 N at A , 80 N at B
(d) 80 N at A , 40 N at B

[ESE : 2002]

- 1.5 A bar of length L tapers uniformly from diameter 1.1 D at one end to 0.9 D at the other end. The elongation due to axial pull is computed using mean diameter D . What is the approximate error in computed elongation?

- (a) 10% (b) 5%
(c) 1% (d) 0.5% [ESE : 2004]

- 1.6 A solid uniform metal bar of diameter D and length L is hanging vertically from its upper end. The elongation of the bar due to self weight is

- (a) Proportional to L and inversely proportional to D^2
(b) Proportional to L^2 and inversely proportional to D^2
(c) Proportional to L but independent of D
(d) Proportional to L^2 but independent of D

[ESE : 2005]

- 1.7 In a tensile test, near the elastic limit zone

- (a) tensile stress increases at a faster rate
(b) tensile stress decreases at a faster rate
(c) tensile stress increases in linear proportion to the stress
(d) tensile stress decreases in linear proportion to the stress

[ESE : 2006]

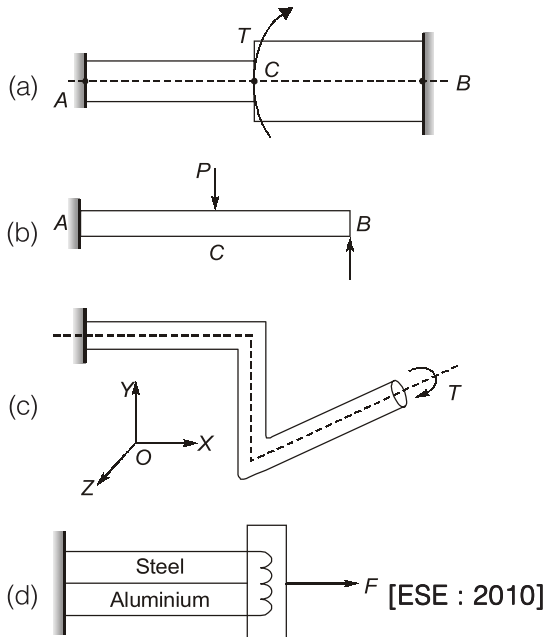
1.8 Two tapering bars of the same material are subjected to a tensile load P . The lengths of both the bars are the same. The larger diameter of each of the bars is D . The diameter of the bar A at its smaller end is $D/2$ and that of the bar B is $D/3$. What is the ratio of elongation of the bar A to that of the bar B ?

- (a) 3 : 2 (b) 2 : 3
(c) 4 : 9 (d) 1 : 3 [ESE : 2006]

1.9 Which one of the following expresses the total elongation of a bar of length L with a constant cross-section of A and modulus of elasticity E hanging vertically and subject to its own weight W ?

- (a) $\frac{WL}{AE}$ (b) $\frac{WL}{2AE}$
(c) $\frac{2WL}{AE}$ (d) $\frac{WL}{4AE}$ [ESE : 2007]

1.10 Which one of the following is not a statically indeterminate structure?



Directions: The following items consists of two statements; one labelled as 'Assertion (A)' and the other as 'Reason (R)'. You are to examine these two statements carefully and select the answers to these items using the codes given below:

Codes:

- (a) both A and R are true and R is the correct explanation of A
(b) both A and R are true but R is not a correct explanation of A

- (c) A is true but R is false
(d) A is false but R is true

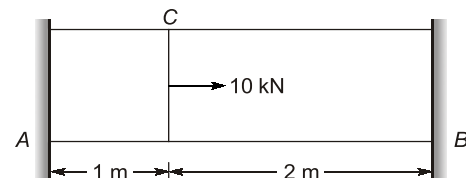
1.11 Assertion (A): A plane state of stress always results in a plane state of strain.

Reason (R): A uni-axial state of stress results in a three-dimensional state of strain. [ESE : 2010]

1.12 Assertion (A): A state of plane strain always results in plane stress conditions.

Reason (R): A thin sheet of metal stretched in its own plane results in plane strain conditions. [ESE : 2010]

1.13 A prismatic bar, as shown in figure is supported between rigid supports. The support reactions will be



- (a) $R_A = \frac{10}{3}$ kN and $R_B = \frac{20}{3}$ kN
(b) $R_A = \frac{20}{3}$ kN and $R_B = \frac{10}{3}$ kN
(c) $R_A = 10$ kN and $R_B = 10$ kN
(d) $R_A = 5$ kN and $R_B = 5$ kN [ESE : 2011]

1.14 A rod of length l tapers uniformly from a diameter D at one end to a diameter d at the other. The Young's modulus of the material is E . The extension caused by an axial load P is

- (a) $\frac{4Pl}{\pi(D^2 - d^2)E}$ (b) $\frac{4Pl}{\pi(D^2 + d^2)E}$
(c) $\frac{4Pl}{\pi DdE}$ (d) $\frac{2Pl}{\pi DdE}$ [ESE : 2012]

1.15 Consider the following statements :

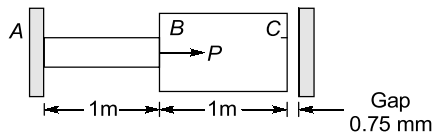
1. State of plane stress occurs at the surface
2. State of plane strain occurs at the surface
3. State of plane stress occurs in the interior part of the plate
4. State of plane strain occurs in the interior part of the plate

Which of these statements are correct?

- (a) 1 and 3 (b) 2 and 4
(c) 1 and 4 (d) 2 and 3

[ESE : 2013]

- 1.16 In the arrangement as shown in the figure, the stepped steel bar ABC is loaded by a load P . The material has Young's modulus $E = 200$ GPa and the two portions AB and BC have area of cross section 1 cm^2 and 2 cm^2 respectively. The magnitude of load P required to fill up the gap of 0.75 mm is



- (a) 10 kN (b) 15 kN
(c) 20 kN (d) 25 kN [ESE : 2013]

- 1.17 A copper rod of 2 cm diameter is completely encased in a steel tube of inner diameter 2 cm and outer diameter 4 cm. Under an axial load, the stress in the steel tube is 100 N/mm^2 .

If $E_s = 2 E_c$, then the stress in the copper rod is

- (a) 50 N/mm^2 (b) 33.33 N/mm^2
(c) 100 N/mm^2 (d) 300 N/mm^2

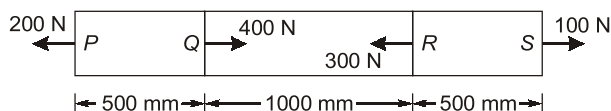
[ESE : 2015]

- 1.18 **Assertion (A):** Tensile strength of CI is much higher than that of MS.

Reason (R): Percentage of carbon in CI is more than 1.5.

- (a) Both A and R are true and R is the correct explanation of A
(b) Both A and R are true but R is not a correct explanation of A
(c) A is true but R is false
(d) A is false but R is true [ESE : 2015]

- 1.19 A steel rod of cross-sectional area 10 mm^2 is subjected to loads at points P , Q , R and S as shown in the figure below :



If $E_{\text{steel}} = 200$ GPa, the total change in length of the rod due to loading is

- (a) $-5 \mu\text{m}$ (b) $-10 \mu\text{m}$
(c) $-20 \mu\text{m}$ (d) $-25 \mu\text{m}$

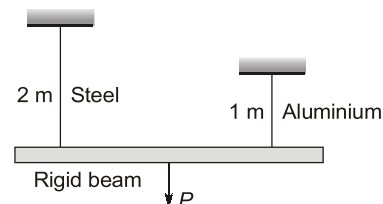
[ESE : 2016]

- 1.20 Two steel rods of identical length and material properties are subjected to equal axial loads. The

first rod is solid with diameter d and the second is a hollow one with external diameter D and internal diameter 50% of D . If the two rods experience equal extensions, the ratio of $\frac{d}{D}$ is

- (a) $\frac{3}{4}$ (b) $\frac{\sqrt{3}}{2}$
(c) $\frac{1}{2}$ (d) $\frac{1}{4}$ [ESE : 2016]

- 1.21 A rigid beam of negligible weight, is supported in a horizontal position by two rods of steel and aluminium, 2 m and 1 m long, having values of cross-sectional areas 100 mm^2 and 200 mm^2 , and Young's modulus of 200 GPa and 100 GPa, respectively. A load P is applied as shown in the figure below:



If the rigid beam is to remain horizontal, then

- (a) the force P must be applied at the centre of the beam
(b) the force on the steel rod should be twice the force on the aluminium rod
(c) the force on the aluminium rod should be twice the force on the steel-rod
(d) the forces on both the rods should be equal

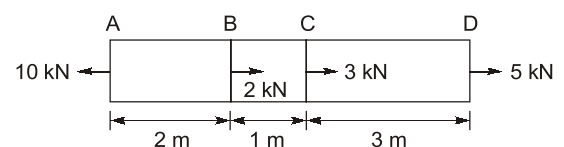
[ESE : 2018]

- 1.22 The resilience of steel can be found by integrating stress-strain curve up to the

- (a) ultimate fracture point
(b) upper yield point
(c) lower yield point
(d) elastic point

[ESE : 2018]

- 1.23 The loads acting on a 3 mm diameter bar at different points are as shown in the figure:



If $E = 205$ GPa, the total elongation of the bar will be nearly

- (a) 29.7 mm (b) 25.6 mm
(c) 21.5 mm (d) 17.4 mm

[ESE : 2019]

1.24 In a propeller shaft, sometimes apart from bending and twisting, end thrust will also develop stresses which would be

- (a) tensile in nature and uniform over the cross-section
(b) compressive in nature and uniform over the cross-section
(c) tensile in nature and non-uniform over the cross-section
(d) compressive in nature and non-uniform over the cross-section

[ESE : 2019]

1.25 A copper piece originally 305 mm long is pulled in tension with a stress of 276 MPa. If the deformation is entirely elastic and the modulus of elasticity is 110 GPa, the resultant elongation will be nearly

- (a) 0.43 mm (b) 0.54 mm
(c) 0.65 mm (d) 0.77 mm

[ESE : 2019]

1.26 A 1.25 cm diameter steel bar is subjected to a load of 2500 kg. The stress induced in the bar will be

- (a) 200 MPa (b) 210 MPa
(c) 220 MPa (d) 230 MPa

[ESE : 2020]

1.27 A 13 mm diameter tensile specimen has 50 mm gauge length. If the load corresponding to the 0.2% offset is 6800 kg, the yield stress will be nearly

- (a) 31 kg/mm² (b) 43 kg/mm²
(c) 51 kg/mm² (d) 63 kg/mm²

[ESE : 2020]

1.28 The linear relationship between stress and strain for a bar in simple tension or compression is expressed with standard notations by the equation

- (a) $\sigma = E\varepsilon$ (b) $\sigma = E\nu$
(c) $\sigma = G\nu$ (d) $\sigma = G\varepsilon$ [ESE : 2020]

■■■■

Answers Stress and Strain

- 1.1 (a) 1.2 (a) 1.3 (b) 1.4 (d) 1.5 (c) 1.6 (d) 1.7 (b) 1.8 (b) 1.9 (b)
1.10 (c) 1.11 (d) 1.12 (d) 1.13 (b) 1.14 (c) 1.15 (c) 1.16 (b) 1.17 (a) 1.18 (d)
1.19 (d) 1.20 (b) 1.21 (c) 1.22. (d) 1.23 (a) 1.24 (b) 1.25 (d) 1.26 (a) 1.27 (c)
1.28 (a)

Explanations Stress and Strain

1.1 (a)

$$A \times \sigma = F$$

$$(W - 10) \times 2 \times 200 = 2000$$

$$\therefore W - 10 = 5$$

$$\therefore W = 15 \text{ mm}$$

1.2 (a)

No stress will be developed in the direction of thickness i.e. $\sigma_{zz} = 0$.

1.3 (b)

For rigid beam is to remain horizontal

$$(\delta l)_{Cu} = (\delta l)_{St.}$$

$$\frac{F_{Cu} \times 1}{2 \times 100} = \frac{F_{St.} \times 2}{1 \times 200}$$

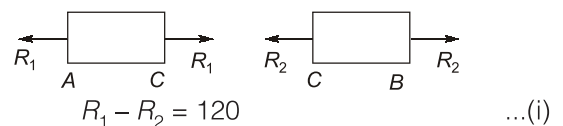
$$F_{Cu} = 2F_{St.}$$

1.4 (d)

$$R_A = 120 \times (BC/AB) = 80 \text{ N/mm}^2$$

$$R_B = 120 \times AC/AB = 40 \text{ N/mm}^2.$$

Free body diagrams,



$$\text{and } (\delta l)_1 + (\delta l)_2 = 0$$

$$\frac{R_1 \times l}{A \times E} + \frac{R_2 \times 2l}{A \times E} = 0$$

$$\therefore R_1 = -2R_2 \quad \dots(ii)$$

From Equation (i) and (ii), we get

$$R_2 = -40 \text{ N}$$

$R_2 = 40 \text{ N}$ (opposite direction to our assumption)

and $R_1 = 80 \text{ N}$

1.5 (c)

Equivalent diameter of the bar

$$= \sqrt{1.1D \times 0.9D} = \sqrt{0.99} D$$

$$\text{Elongation } (\Delta l_2) = \frac{4F}{E \times \pi \times 0.99 D^2}$$

$$\text{Elongation due to mean diameter} = \frac{4F}{E \times \pi D^2}$$

\therefore Percentage error,

$$= \frac{\frac{4F}{E \pi D^2} \left[\frac{1}{0.99} - 1 \right]}{\frac{4F}{E \pi D^2} \times 0.99} \times 100\% = 1\%$$

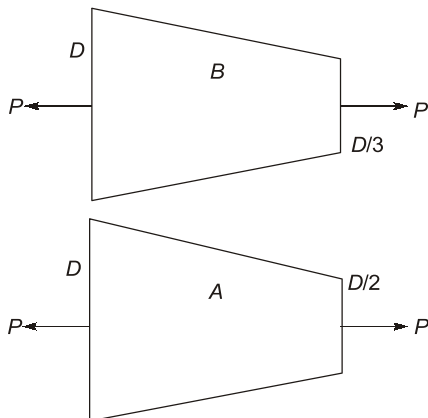
1.6 (d)

$$\text{Elongation due to self weight} = \frac{\gamma L^2}{2E}$$

1.8 (b)

$$\text{Elongation of taper bar} = \frac{4PL}{E \pi d_1 d_2}$$

$$\delta l \propto \frac{1}{d_1 d_2}$$



$$\frac{(\delta l)_A}{(\delta l)_B} = \frac{D \times \frac{D}{3}}{D \times \frac{D}{2}} = \frac{2}{3}$$

1.9 (b)

Deflection of elemental length ' dx '

$$d\Delta = \frac{W_x \cdot dx}{AE}$$

Total deflection

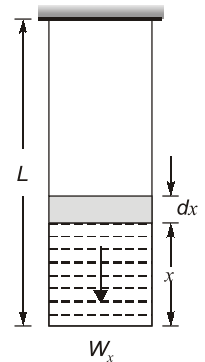
$$\Delta = \int_0^L \frac{W_x \cdot dx}{AE}$$

$$W_x = V_x \cdot \gamma = A \cdot x \cdot \gamma$$

where γ = Weight density of metal

$$\Delta = \int_0^L \frac{\gamma \cdot A \cdot x \cdot dx}{AE} = \frac{\gamma L^2}{2E} = \frac{W}{V} \times \frac{L^2}{2E}$$

$$\Delta = \frac{W}{A \times L} \times \frac{L^2}{2E} = \frac{WL}{2AE}$$



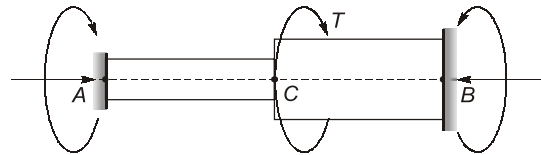
1.10 (c)

Degree of static indeterminacy of a plane structure

$$(D_s) = R_e - 3$$

where R_e is number of external reactions.

Free body diagram for (a) is



Degree of static indeterminacy for above figure is

In this case

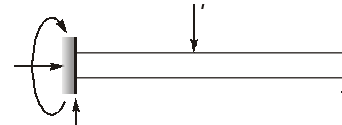
$$R_e = 6$$

$$\text{hence } D_s = 6 - 3 = 3$$

$$\text{Since } D_s > 0$$

it is statically indeterminate structure.

Free body diagram is (b) is



Degree of static indeterminacy for above figure is

In this case

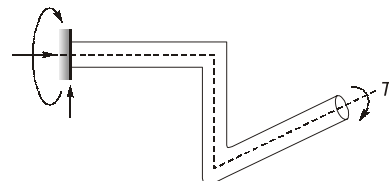
$$R_e = 4$$

$$\text{hence } D_s = 4 - 3 = 1$$

$$\text{Since } D_s > 0$$

it is statically indeterminate structure.

Free body diagram is (c) is



Degree of static indeterminacy for above figure is

In this case $R_e = 3$

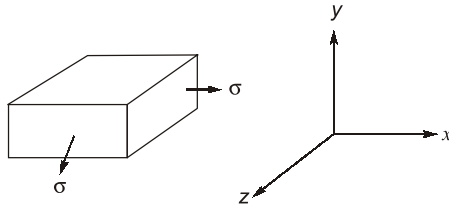
hence $D_s = 3 - 3 = 0$

Since $D_s = 0$

it is not statically indeterminate structure.

1.11 (d)

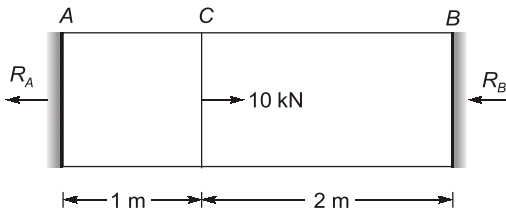
A plane state of stress does not result in a plane state of strain



$$\epsilon_x = \frac{\sigma}{E}, \epsilon_y = -\frac{\mu\sigma}{E}, \epsilon_z = -\frac{\mu\sigma}{E}$$

1.13 (b)

Let us consider reaction R_A and R_B at point A and B in direction opposite to that of 10 kN. We are just assuming the direction. If value of reaction came out to be negative then just take the direction opposite to that of assumed direction.



By using equilibrium condition, we have

$$R_A + R_B = 10 \quad \dots(i)$$

By using deflection condition

$$\Delta_{AC} + \Delta_{CB} = 0$$

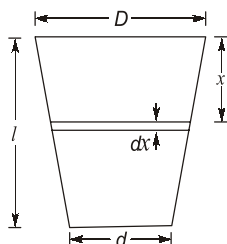
$$\frac{R_A \times 1}{AE} + \frac{(R_A - 10) \times 2}{AE} = 0$$

$$R_A \times 1 + 2R_A - 20 = 0$$

$$R_A = \frac{20}{3} \text{ kN}$$

$$\therefore R_B = \frac{10}{3} \text{ kN}$$

1.14 (c)



$$\text{Area of elementary ring} = \pi r^2 = \frac{\pi}{4} d_x^2$$

$$d_x = ax + D$$

$$d = al + D$$

$$a = \frac{d - D}{l}$$

$$\text{Elongation of element} = \left(\frac{\sigma}{E} \right) \times dx$$

$$= \frac{P}{AE} dx = \frac{P}{E \frac{\pi}{4} d_x^2} dx$$

$$\text{Total elongation} = \int_0^l \frac{4P}{\pi E} \frac{dx}{(ax + D)^2}$$

$$= \frac{4P}{\pi E} \left[-\frac{1}{ax + D} \times \frac{1}{a} \right]_0^l$$

$$= \frac{4P}{\pi E} \left(-\frac{1}{a} \right) \left[\frac{1}{al + D} - \frac{1}{D} \right]$$

$$= \frac{4P}{\pi E} \left(-\frac{1}{a} \right) \left[\frac{1}{\frac{d-D}{l} l + D} - \frac{1}{D} \right]$$

$$= \frac{4P}{\pi E} \left(-\frac{1}{\frac{d-D}{l}} \right) \left(\frac{D-d}{dD} \right) = \frac{4Pl}{\pi dDE}$$

1.15 (c)

Plane stress condition for thin plates.

Plane strain condition for thick plates.

1.16 (b)

$$\Delta_{\text{total}} = \Delta_{AB} = 0.75 \text{ mm}$$

$$\therefore \left(\frac{PL}{AE} \right)_{AB} = 0.75 \text{ mm}$$

(since $\Delta_{BC} = 0$)

where, $L_{AB} = 1000 \text{ mm}$; $A_{AB} = 1 \text{ cm}^2 = 100 \text{ mm}^2$

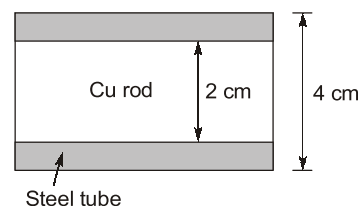
$$E = 200 \times 10^3 \text{ MPa}$$

$$\frac{P \times 1000}{100 \times 200 \times 10^3} = 0.75 \text{ mm}$$

$$P = 15000 \text{ N}$$

$$P = 15 \text{ kN}$$

1.17 (a)



Given:

$$\begin{aligned}\sigma_s &= 100 \text{ MPa} \\ E_s &= 2E_c \\ \delta_s &= \delta_c \\ \frac{P_s L_s}{A_s E_s} &= \frac{P_c L_c}{A_c E_c} \\ \frac{\sigma_s}{E_s} &= \frac{\sigma_c}{E_c} \quad [\because L_s = L_c] \\ \sigma_c &= \frac{\sigma_s}{E_s} \times E_c \\ \sigma_c &= \frac{\sigma_s}{2} \\ \sigma_c &= \frac{100}{2} = 50 \text{ MPa}\end{aligned}$$

1.18 (d)

Since tensile strength of MS is much higher than cast iron, Statement I is wrong. Cast Iron is strong in compression.

Statement II is correct.

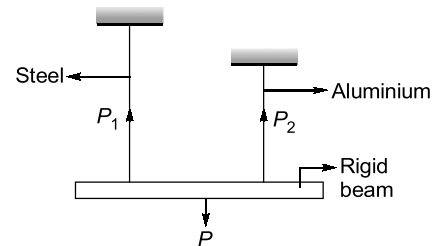
1.19 (d)

$$\begin{aligned}\Delta &= \Delta_{PQ} + \Delta_{QR} + \Delta_{RS} \\ &= \frac{F_{PQ} L_{PQ}}{AE} + \frac{F_{QR} L_{QR}}{AE} + \frac{F_{RS} L_{RS}}{AE} \\ &= \frac{1}{AE} [(200 \times 0.5) + (-200 \times 1) + (100 \times 0.5)] \\ &= \frac{100 - 200 + 50}{10 \times 10^{-6} \times 200 \times 10^9} = -25 \times 10^{-6} \text{ m} \\ &= -25 \mu\text{m}\end{aligned}$$

1.20 (b)

$$\begin{aligned}\Delta_s &= \Delta_H \\ \frac{PL}{A_s \cdot E} &= \frac{PL}{A_H \cdot E} \\ \therefore A_s &= A_H \\ d^2 &= D^2 - \left(\frac{D}{2}\right)^2 \\ d^2 &= \frac{3D^2}{4} \\ \therefore \frac{d}{D} &= \frac{\sqrt{3}}{2}\end{aligned}$$

1.21 (c)

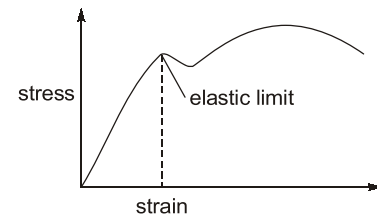


If the rigid beam is to remain horizontal

$$\begin{aligned}(\delta_L)_1 &= (\delta_L)_2 \\ \frac{P_1 L_1}{A_1 E_1} &= \frac{P_2 L_2}{A_2 E_2} \\ \frac{P_1 \times 2000}{100 \times 200 \times 10^3} &= \frac{P_2 \times (1000)}{200 \times 100 \times 10^3} \\ P_2 &= 2P_1 \text{ [i.e. } P_{Al} = 2P_{steel}] \end{aligned}$$

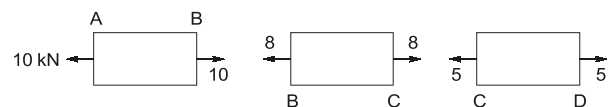
1.22 (d)

The resilience of steel can be found by integrating stress-strain curve upto elastic point.

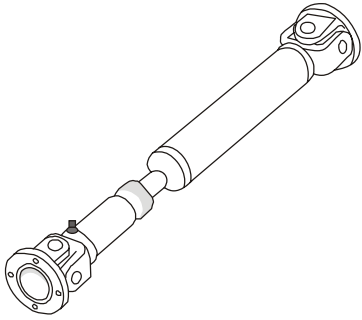


Resilience involves ability to absorb energy by a material upto elastic limit.

1.23 (a)



$$\begin{aligned}\Delta &= \Delta_{AB} + \Delta_{BC} + \Delta_{CD} \\ &= \frac{10 \times 10^3 \times 2000}{AE} + \frac{8 \times 10^3 \times 1000}{AE} \\ &\quad + \frac{5 \times 10^3 \times 3000}{AE} \\ &= \frac{43 \times 10^6}{AE} = \frac{43 \times 10^6}{\frac{\pi}{4} \times 3^2 \times 205 \times 10^3} \\ &= 29.68 \text{ mm}\end{aligned}$$

1.24 (b)

Propeller shaft is under compression due to thrust in X-Y.

1.25 (d)

$$\delta L = \frac{PL}{AE} \text{ or } \frac{\sigma_a L}{E} = \frac{276 \times 305}{110 \times 10^3}$$

$$= 0.765 \text{ mm} \simeq 0.77 \text{ mm}$$

1.26 (a)

$$\text{Axial stress, } \sigma = \frac{P}{A} = \frac{2500 \times 9.81}{\frac{\pi}{4} (12.5)^2} = 200 \text{ MPa}$$

1.27 (c)

$$\text{Yield stress, } \sigma = \frac{\text{Load}}{\text{Area}} = \frac{6800}{\frac{\pi}{4} \times (13)^2}$$

$$= 51.25 \text{ kg/mm}^2$$

■■■■